Ductility of timber connections - discussion on the base of examples

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Joint ductility – example 1: dowel type shear connection

Evaluation of cyclic response of wall-to-wall screwed connection in CLT construction system

One monotonic and 5 quasi-static reversed-cyclic tests according ISO 16670 (following also requirements in EN 12512)

Structure of joint - two CLT wall segments mechanically connected by 3 screws 8,0x160/80 (Würth Eco Assy II screw)





UL FGG Ljubljana

Norway Trondheim, 2009, 26-27, March ົບ LO -5 Ö



Norway **Comparison with similar tests on CLT** 20 March 26-27, 2009, Trondheim, Results from UK (TH) repor Nr. 016108 Used screws in UK (TH) tests: 18 R 12 S (8,0x200/120, d_s=5,7mm) R 13 S (8,0x300/120 d_s=5,7mm) 16 14 12 10



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Discussion – example 1

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- Big difference in tests results comparing visibly just slight difference in specimens configurations
- Important changing parameter was the length of the treated shank and its position in the shear layer
- In calculation the most important parameter is the "rope effect"
- Initial friction between members is not taken into account but importantly influence on test results
- Definition of ductility is questionable from monotonic results as it returns higher values comparing evaluation from cyclic response
- Impairment of strength in repeated cycles has to be recognized as important parameter for definition of static ductility

Structural ductility – example 2: shear response of timber frame wall



Structural ductility – example 2: shear response of timber frame wall





Discussion – example 2, 3

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- Difference in tests results between monotonic and cyclic response
- System could behave ductile even if it includes main construction elements with brittle behavior
- If construction system is composed of different elements, ductile element have to prevent with their capability of non-linear (plastic) behavior failure of brittle ones (method of "fuse")
- In the system dissipative zones have to be carefully designed as ductile construction parts
- Brittle and/or non-ductile parts of construction have to be designed with overstrengths
- Ductile element have to be weaker than non-ductile elements – principle of the weakest element in chain



General conclusions

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- How to define ductility on the base of monotonic and cyclic test?
- Load K., KEN594 F_{max}, d_{Fmax} F_{1%h} $-F_u$ F_{0.75%h} F_{0.5%h} 0.9 F_{max} 0.8 Fmax Fv 0.4 F_{max} 0.1 F_{max} d_{r 0.75%h} Displacement d_v d_{r 0.5%h} d_{r 1%h} du

- What is a different?
- How could be used monotonicaly defined ductility in seismic engineering?
- For definition of ductility the basic parameters as evaluation of deformation at the end of elastic behavior and definition of ultimate deformation for different responses have to be agreed!

